PARADISA HOMES

7427 NORTH LAMAR BLVD STE 101 AUSTIN, TX 78752 512.910.4505

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3105 WHITEPINE DR paradisahomes

AUSTIN, TX 78757

DOOR NOTES:

- 1. ALL GLAZING IN DOORS SHALL BE TEMPERED.
- 2. COORDINATE KEYING AND HARDWARE FOR DOORS WITH OWNER.

3. ALL EXTERIOR DOORS SHALL BE FULLY WEATHER STRIPPED.

- 4. ALL HAND ACTIVATED DOOR OPENING HARDWARE SHALL BE MOUNTED 34" TO 48" ABOVE FINISH FLOOR.
- 5. INTERIOR DOOR UNDERCUTS SHALL NOT EXCEED 3/4".
- 6. PROVIDE A CAULK BEAD AT THE BASE OF EACH DOOR FRAME THAT TERMINATES AT ANY NON-CARPET FLOORING.

WINDOW NOTES:

- 1. GLAZING WITHIN 24" OF EITHER SIDE OF ANY DOOR SHALL BE
- 2. GLAZING GREATER THAN 9 S.F. IN AREA WITH A BOTTOM EDGE LESS THAN 18" ABOVE (AND HORIZONTALLY WITH IN 36") OF A WALKING SURFACE SHALL BE TEMPERED.
- 3. ALL WINDOWS TO BE DOUBLE PANE, INSULATED, LOW-E WITH AN ARGON FILLING
- 4. AT LEAST ONE WINDOW IN EACH BEDROOM SHALL BE 44" AFF MAX. AND OPEN 20" WIDE MIN. BY 24" HIGH MIN. AND OPEN 5.7 SQ. FT. COORDINATE EGRESS WITH ACTUAL MANUFACTURER AS EACH VARIES.

FIRE PROTECTION

- PROVIDE SMOKE ALARMS- HARD WIRED, INTERCONNECTED, BATTERY BACKUP AT EACH SLEEPING ROOM AND IMMEDIATE COMMON AREA OUTSIDE OF SLEEPING ROOMS AND LOCATED AT NOT LESS THAN 3FT. FROM A DOOR TO A BATHROOM WITH TUB OR SHOWER EXCEPT WHEN THIS REQUIREMENT WILL PREVENT THE INSTALLATION OF A SMOKE ALARM IN A REQUIRED LOCATION, AND IF APPLICABLE, ON EACH ADDITIONAL STORY INCLUDING BASEMENTS AND HABITABLE
- 2. PROVIDE CARBON MONOXIDE ALARM-HARD WIRED WITH BATTERY BACKUP, INSTALLED OUTSIDE OF EACH SEPARATE SLEEPING AREA IN THE IMMEDIATE VICINITY OF THE BEDROOMS IN DWELLING UNITS WITHIN WHICH FUEL-FIRED APPLIANCES ARE INSTALLED AND/OR HAVE ATTACHED GARAGE.
- 3. ALL EAVES WITHIN 5'-0" OF PROPERTY LINES ARE TO BE OF FIRE RATED CONSTRUCTION BY PROVIDING BLOCKING FROM TOP PLATE TO UNDERSIDE OF ROOF WHERE EAVES IS WITHIN 5'-0" OF PROPERTY LINE.

APPLICABLE CODES:

- 1. 2021 INTERNATIONAL BUILDING CODE (IBC)
- 2. 2021 INTERNATIONAL RESIDENTIAL CODE (IRC)
- 3. 2021 UNIFORM PLUMBING CODE (UPC)
- 4. 2021 UNIFORM MECHANICAL CODE (UMC) 5. 2020 NATIONAL ELECTRICAL CODE (NEC)

ARCHITECTURAL

- A1.0 EXISTING & PROPOSED SITE PLAN

- A2.1 2ND FLOOR PLAN
 A2.2 ROOF PLAN
 A2.3 VISITABILITY PLAN
 A3.0 1ST ELECTRICAL PLAN
 A3.1 2ND ELECTRICAL PLAN
 A4.0 ELEVATIONS
- S301 ROOF FRAMING PLAN
 S302 1ST FLOOR WALL BRACING PLAN
 S303 2ND FLOOR WALL BRACING PLAN S400 FRAMING DETAILS

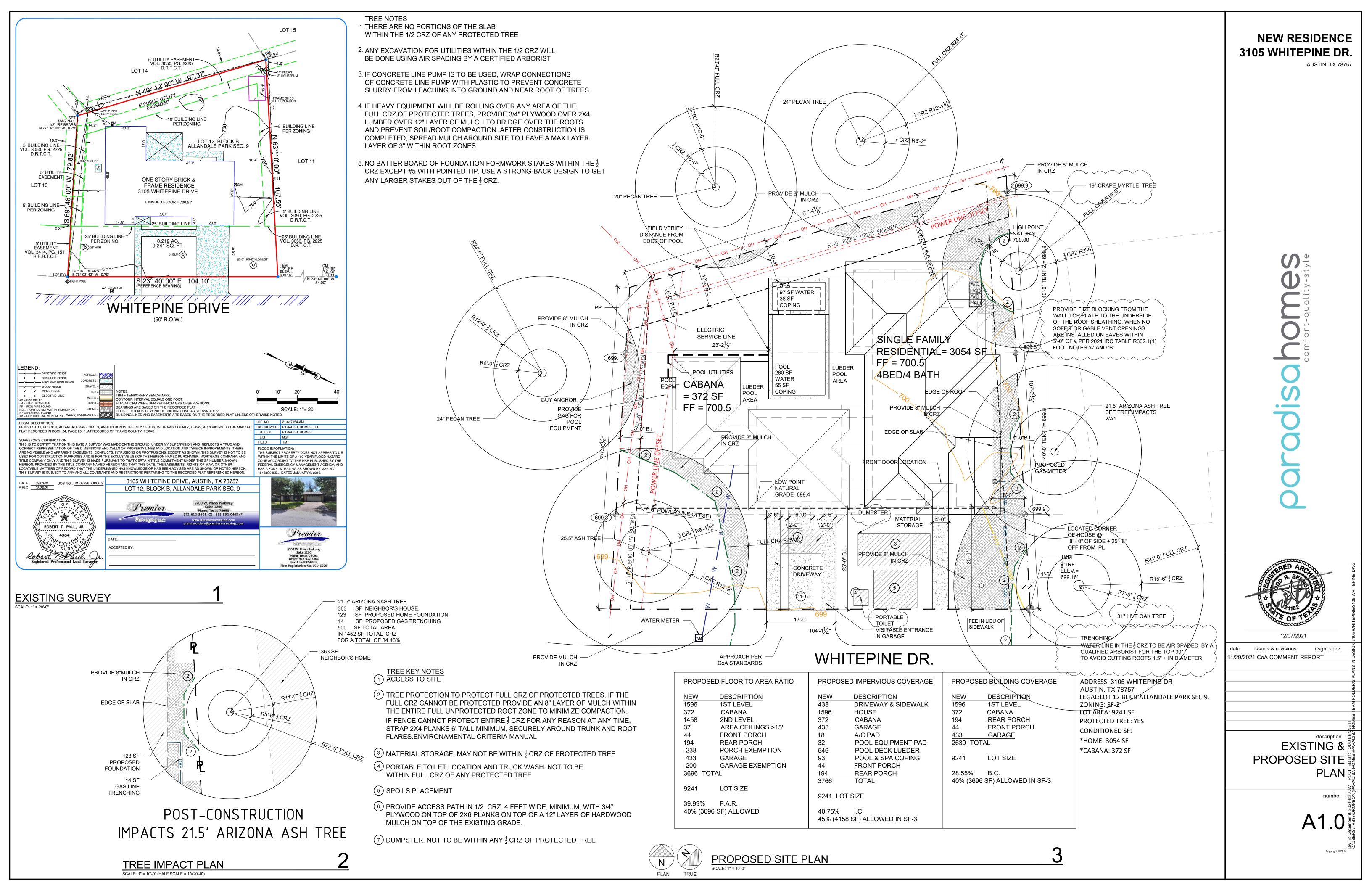
AUSTIN VISITABILTY

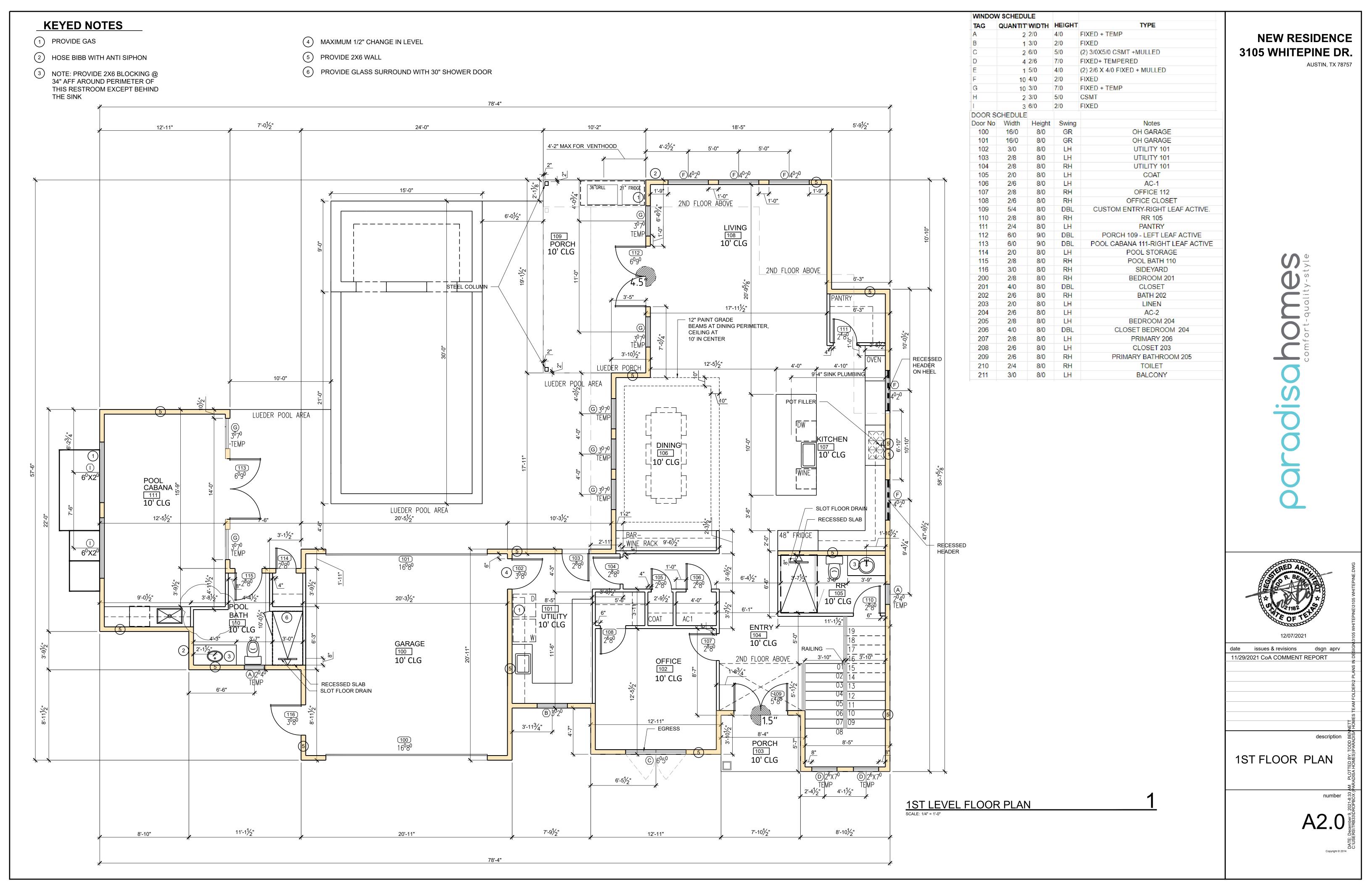
- ALL SWITCHES AND TEMPERATURE CONTROLS ARE TO BE MOUNTED 48" AFF MAXIMUM
- ALL OUTLETS ARE TO BE MOUNTED 15" AFF MINIMUM
- LEVEL ONE MUST HAVE A 32" CLEAR PATH FROM DESIGNATED VISITABLE ENTRANCE TO LEVEL 1 RESTROOM, KITCHEN, LIVING AND DINING.
- PROVIDE A MINIMUM 32" WIDE DOOR TO ALLOW 30" CLEAR @ BATHROOM DOORWAY

 PROVIDE 2x6 BLOCKING IN DESIGNATED LEVEL ONE VISITABILITY RESTROOM AROUND PERIMETER OF ROOM @ 34" AFF TO CENTER

 ONE ENTRY INTO LEVEL ONE MUST HAVE A CHANGE IN LEVEL OF NO MORE THAN 1/2" MAX AND PROVIDE 32" CLEAR @ DOORWAY

3105 WHITEPINE AUSTIN, TX 78757

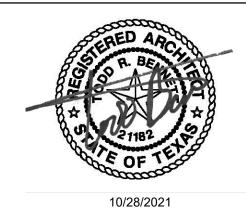




NEW RESIDENCE 3105 WHITEPINE DR.

AUSTIN, TX 78757

porolisa homes comfort-quality-style



date issues & revisions dsgn aprv

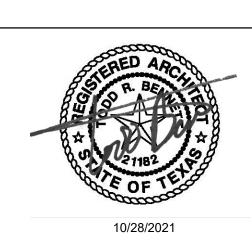
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2ND FLOOR PLAN

number

A2.1

2ND LEVEL FLOOR PLAN
SCALE: 1/4" = 1'-0"



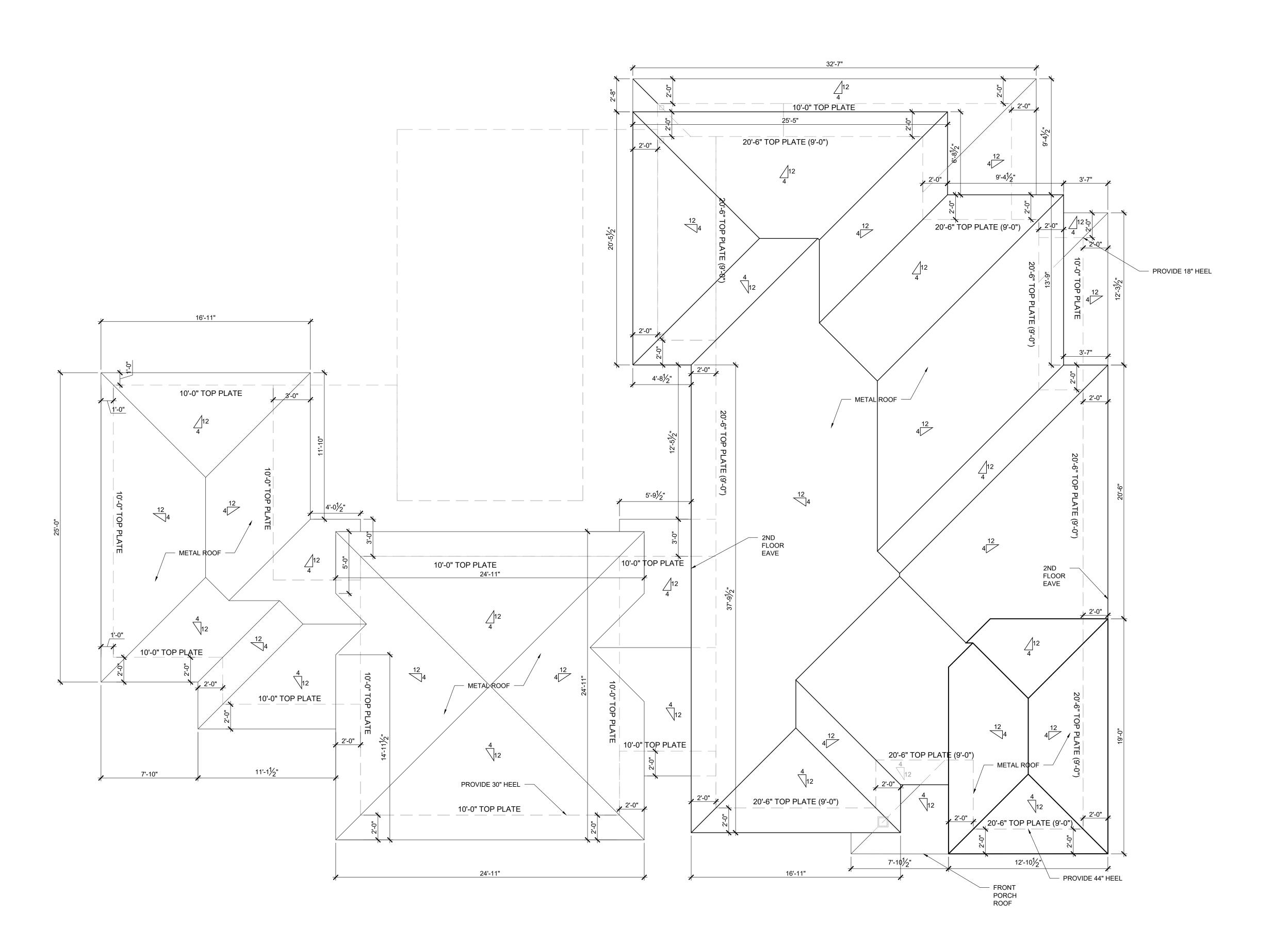
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description

ROOF PLAN

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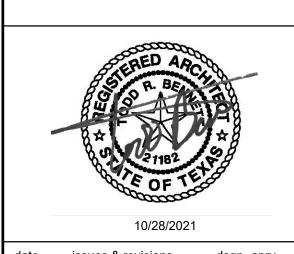
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NEW RESIDENCE 3105 WHITEPINE DR.

AUSTIN, TX 78757

paralle comfort-quality-style



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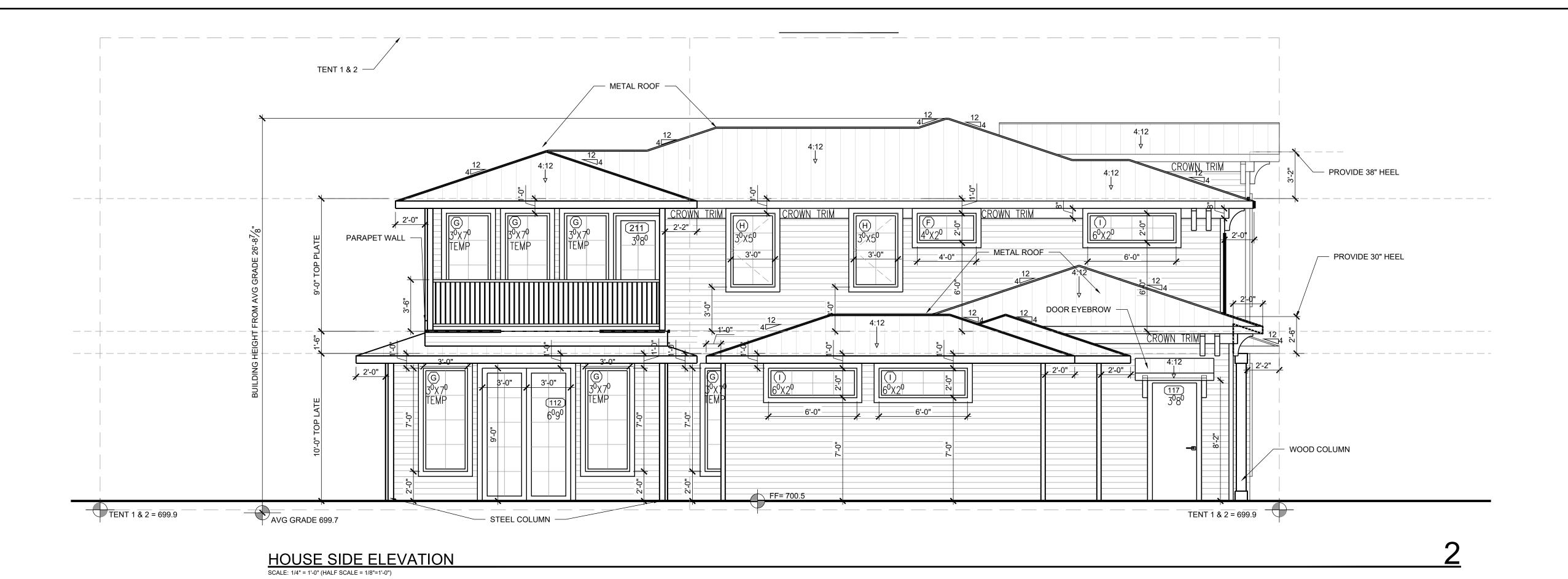
VISITABILITY PLAN

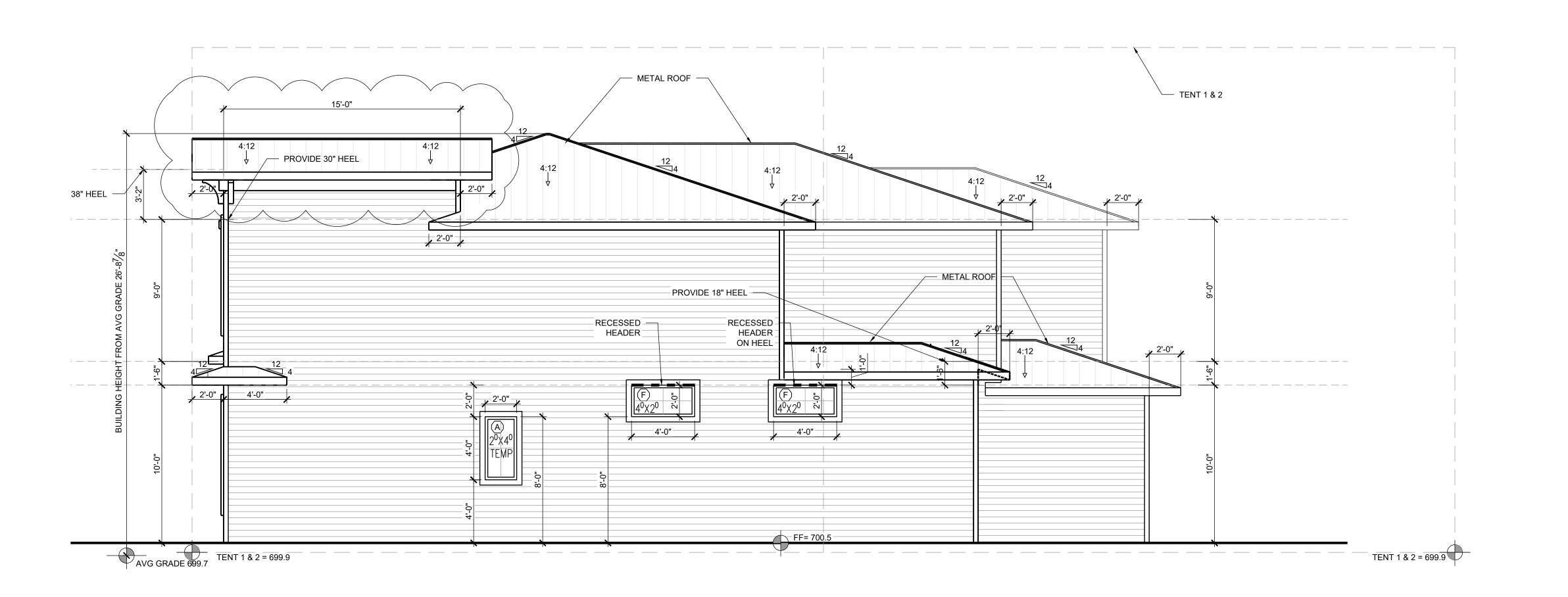
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A2.3







HOUSE SIDE ELEVATION

SCALE: 1/4" = 1'-0" (HALF SCALE = 1/8"=1'-0")

NEW RESIDENCE 3105 WHITEPINE DR.

AUSTIN, TX 78757

Odrogen Salandes Comfort-quality-style



date issues & revisions dsgn aprv
11/29/2021 CoA COMMENT REPORT

description

ELEVATIONS

^ ^ ^ ^

A4.1

M

CHK. BY: TZ DRWN. BY: BB

DATE: 10/29/2021

SHEET NO.

S100

78**'**–4" 5'-9½" 7'-0½" 12'-11" 24**'**-0" 28**'**-7" 14'-41/4" 9'-73/4" 8'-10½" 9'-6½" 10'-2" S200 SL₱ 1/8"/FT (S200) (1 (S200) 4" CONC SLAB 4" CONC SLAB 1B S200 TYP (NOTE 2) TYP (NOTE 2) INT BM $\binom{2}{S200}$ 4----RB . S200 I VARIES 4" CONC SLAB TYP (NOTE 2) RB S200 S200 / --// S200 RB (S200) _NOTE 9 (1B) (S200) S200 S200 S200 C1 (NOTE 7)—— 8'-0½" 14'-41/4" 6'-63/4" 7'-10½" 8'-10½" 3'-1"

7'-9½"

78**'**–4"

12'-11"

16'-9"

20'-11"

11'-1½"

8'-10"

TENDON ELONGATION SCHEDULE LENGTH | ELONGATION | LENGTH | ELONGATION | (FEET) (INCHES) (FEET) (INCHES) (INCHES) 9.3 12 0.9 66 5.2 120 9.5 14 1.1 68 5.4 122 9.6 16 1.3 70 5.5 124 9.8 18 72 5.7 126 10.0 1.4 20 1.6 74 5.8 128 10.1 22 1.7 76 6.0 130 10.3 24 1.9 78 6.2 132 10.4 26 2.1 80 6.3 10.6 28 2.2 82 6.5 136 10.7 30 2.4 84 6.6 138 10.9 32 2.5 86 6.8 140 11.1 34 2.7 7.0 88 11.2 2.8 90 7.1 11.4 7.3 3.0 92 146 11.5 7.4 40 3.2 94 148 11.7 42 3.3 96 7.6 150 11.9 44 7.7 12.0 3.5 98 46 3.6 100 7.9 154 12.2 48 3.8 102 156 12.3 8.1 50 4.0 104 8.2 158 12.5 52 106 160 12.6 4.1 8.4 4.3 108 8.5 162 12.8 110 56 4.4 8.7 13.0 58 4.6 112 8.8 13.1 60 114 13.3 9.0 116 9.2 170 4.9

FOUNDATION PLAN

1/4" = 1'-0" IN 22X34 1/8" = 1'-0" IN 11X17

PLAN NOTES:

- 1. VERIFY WITH ARCHITECTURAL DRAWINGS FOR DIMENSIONS, SLOPES, SLAB DROPS & FLOOR DRAINS.
- 2. 4" SLAB (U.N.O.) OVER 10 MIL STEGO VAPOR RETARDER OVER COMPACTED STRUCTURAL FILL. REINFORCÈ SLAB WITH 1/2" POST-TENSIONED TENDONS. TENDONS SHALL SPACE NO LESS THAN 36" O.C. AND NO MORE THAN 48" O.C UNLESS NOTED OTHERWISE IN PLAN. FIRST TENDON(S) SHALL BE PLACED BETWEEN 6" TO 18" OF THE PARALLEL CONCRETE EDGE.

THE POST-TENSIONING CONTRACTOR SHALL PREPARE A LOG THAT DOCUMENTS THE ELONGATIONS OF EACH CABLE AND SUBMIT FOR ENGINEER'S REVIEW.

- 3. 2-#3x4'-0" CORNER BARS TYPICAL AT ALL RE-ENTRANT CORNERS.
- 4. RB = REINFORCED BEAM. SEE DETAIL FOR REBARS.
- 5. ENDON DEAD END.
- DESIGNATES TENDON LIVE END. BT = BEAM TENDON; S-32 DESIGNATES "SLAB TENDON AND LENGTH IN FEET"
- 6. POST-TENSIONING SLAB TENDONS SHALL BE IN A STRAIGHT LINE FROM STRESSING END TO DEAD END. DEVIATION AT ANY PLACE SHALL NOT EXCEED 10 DEGREES IN A HORIZONTAL GRADUAL PARABOLIC SWEEP.
- 7. C1 = 8X8 WOOD COLUMN.
- PROVIDE SIMPSON ABU POST BASE w/1/2" EPOXY BOLT (EMBED = 5").
- 8. DOWNSPOUT SHALL NOT EMPTY WATER NEAR FOUNDATION. USE FRENCH DRAIN TO DIRECT WATER AT LEAST 10'-0" AWAY FROM FOUNDATION.
- 9. #3 REBARS AT 12" O.C. AT WHERE THERE IS NO SLAB CABLE.
- 10. C2 = HSS4X4X1/4 STEEL COLUMN. EP = EMBED PLATE. SEE S200 FOR EMBED PLATE DETAILS.

Retained on No. 40 mesh sieve

- Prior to placing fill material, remove all organic and other deleterious material from the existing subgrade for the area within the building line. All exposed surfaces shall then be recompacted to a minimum of 95 percent of the maximum dry density as defined by TxDOT test method TEX 113-E or 114—E at a moisture content within 3 percent of the optimum moisture content. This procedure does not apply to the protected tree's 50% CRZ.
- Structural fill shall be placed in 8 inch loose lifts, watered as required and compacted to a minimum of 95 percent of the maximum dry density as defined in TxDOT test method TEX 113-E at a moisture content within 3 percent of the optimum moisture content. THIS PROCEDURE DOES NOT APPLY TO PROTECTED TREE'S 50% CRITICAL ROOT ZONE.
- Provide a 10 mil polyolefin Stego retarder. Place vapor barrier in accordance with manufacturer's recommendation on top of structural fill.

POST-TENSIONED CONCRETE

- 1. Design and construction of post-tensioned concrete shall be in accordance with Post-tensioning Institute. 2. All post-tensioning tendons shall be Lo-Lax and anchorages shall conform to report No. ACI 423-3R-83. Tendons shall be fabricated with 1/2" diameter 270 KSI strand meeting ASTM A-416.
- 3. All concrete shall have a minimum compressive strength of 2,000 PSI at time of full stressing. Water content shall be controlled and minimized otherwise cracking due to shrinkage will be excessive.
- 4. The location of construction joints as detailed in the contract documents may not be changed without approval from engineer.
- 5. Tendons and reinforcing bars shall be tied at all intersections. Tendons shall be supported on chairs at no more than 4 feet on center. Rebars shall be adequately supported. Care shall be used during placement of concrete so that positioning of tendons and supports is maintained.
- 6. At dead ends, tendon sheathing may be cut back as much as 12" from the anchorage. At stressing ends, sheathing may be cut back a maximum of 2". Repair damaged sheathing prior to concrete placement.
- 7. Dead ends and stressing ends shall not be changed in the filed without a written approval from engineer.
- 8. Concrete shall be well consolidated in the vicinity of end anchorage.
- 9. Tendon force variations indicated by gage pressure and elongation in excess of 7 percent shall be reported to the structural engineer.
- 10. Tendons shall be stressed as follows:
- a) Tendons less than 100 feet shall be fully stresses within 3 to 4 days after concrete placement and achieving a compressive strength of 2,000 psi.
- b) Tendons greater than 100 feet shall be partially stressed to 25% of the full stress force within 24 to 36 hours of concrete placement. Full stressing of these tendons shall occur within 3 and 4 days, after concrete has reached a compressive strength of 2,000 psi.

© 24" O.C.

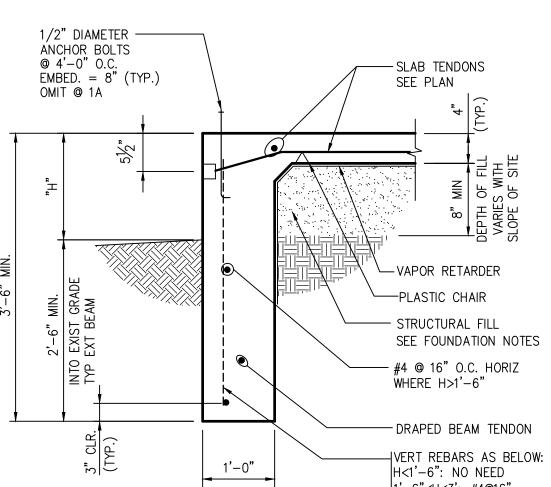
- 11. The jacking force in the 1/2" diameter Lo-lax prestressing strands shall be as follows:
- a) Initial jacking force = 33 kips.
- b) Final jacking force = 31 kips.
- 12. The post-tensioned slab-on-grade design is based on prestressing losses not exceeding 20% or 4.59 kips per tendon excluding seating and frictional losses.
- 13. Tendons 1/2" ϕ and 270 ksi shall be efficiently anchored at 31 kips. These tendons may be temporarily stresses to 33 kips in order to overcome friction and compensate for seating losses.
- 14. Elongations shall be approximately 0.079" per foot of stressed tendon length u.n.o. on plan. Any discrepancies shall be brought to the attention of the engineer.
- 15. Tendons ends shall be cut off with a saw or shear, not a torch.
- 16. Stressing pockets shall be patched with a stiff grout mix.
- 17. Contractor shall submit complete placing plans, details of tendon and bar placements.
- 18. Post-tensioned tendons in beams shall be draped as shown in plans using smooth parabolic drapes.
- 20. After tendons are stressed and excess cut off, any exposed hardware shall be sprayed with rust-inhibitive paint and openings shall be grouted flush with slab edge. 21. Tendons shall have the prestressing strand permanently protected against corrosion by a chemically stable,
- properly applied continuous coating over the entire tendon length. Sheathing for unbonded tendons shall have sufficient tensile strength and water resistance to resist damage and deterioration and shall be continuous over the tendon length.
- 22. Anchorages of unbonded tendons shall develop at least 95% of the minimum specified ultimate strength of the prestressing steel without exceeding anticipated set. Special reinforcement required for the performance of the anchorage shall be provided by the tendon supplier.
- 23. Post—tensioning contractor shall submit the followings to the engineer for review:
- a) Lab test and results on anchorage system.
- b) Current calibration date for stressing equipment to be used.
- c) Coefficient of friction for strands.
- d) Mill tests for strands.
- 24. Post-tensioning slab tendons shall be placed in a straight line from stressing end to dead end. Deviation at any place shall not exceed 10 degrees in a horizontal gradual parabolic sweep.
- 25. tendons shall not be placed within 6" of a parallel edge.
- 26. Contractor shall be responsible for locating all post-tensioning tendons when using expansion bolts, adhesive anchors or power—actuated fasteners. The contractor shall locate the tendons using non—destructive means.

CAST IN PLACE CONCRETE

- 1. Cast in place concrete shall meet the following requirements:
- 28 Day Class Strength Type Size Slump Use A 3000 psi C 33 1" 4" to 6" Slab-on-grade & grade beams
- The use of fly ash is recommended, but shall not exceed 25% of the total of the cement plus fly ash by weight.

CONCRETE REINFORCING

- 1. Reinforcing steel shall be deformed new billet steel bars in accordance with ASTM A615 Grade 60.
- 2. Detailing of reinforcing steel shall conform to the American Concrete Institute Detailing Manual.
- 3. Provide 2-#4 bent bar with 2'-0" legs top and bottom in interior and exterior face of grade beams at corners and top and bottom in exterior face of grade beam at intersections.
- 4. All hooks and bends in reinforcing bars shall conform to ACI detailing standards unless shown otherwise.
- 5. Welding of reinforcing steel will not be permitted.
- 6. Heat shall not be used in the fabrication or installation of reinforcement.
- 7. Reinforcing steel clear cover shall be as follows:
- a. Grade beams 1 1/2" top, 3" bottom, 2" side (formed), 3" side (placed against earth)



DRIVEWAY SLAB - NOT-

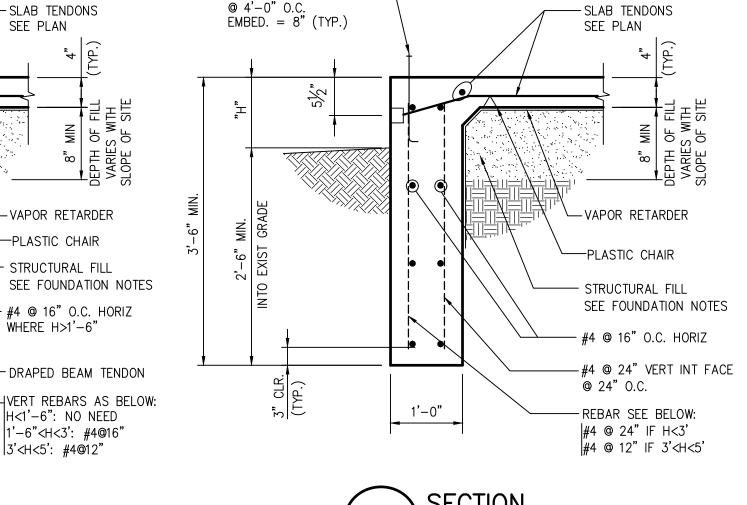
1/2"øx1'-6" SMOOTH —

NOTE: SEE 1/THIS SHEET FOR INFORMATION NOT SHOWN.

DOWELS @ 12" O.C.

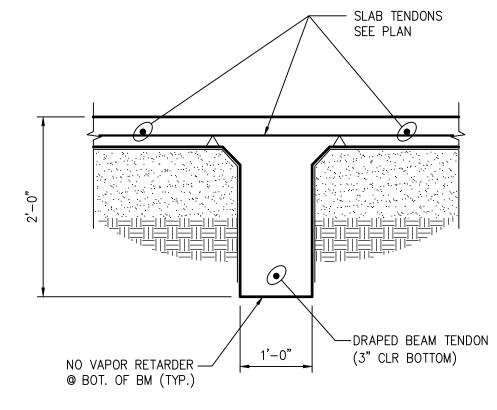
GREASE ONE END

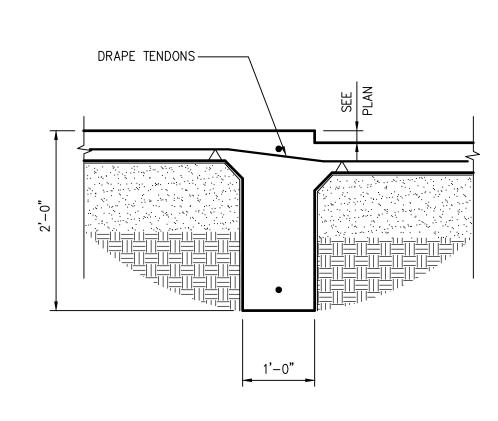
BY GREENEARTH



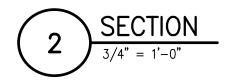
1/2" DIAMETER -

ANCHOR BOLTS

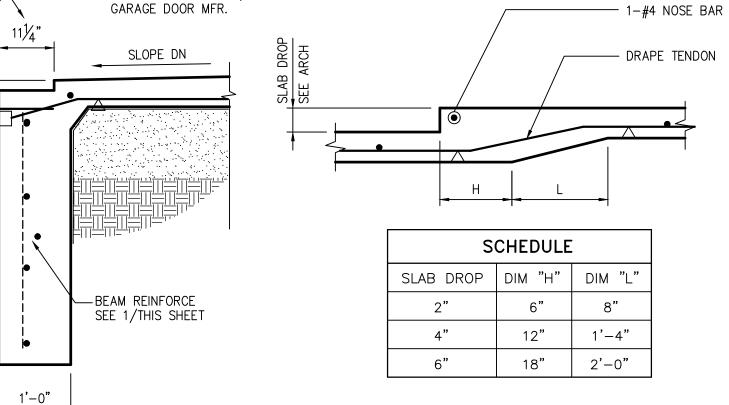


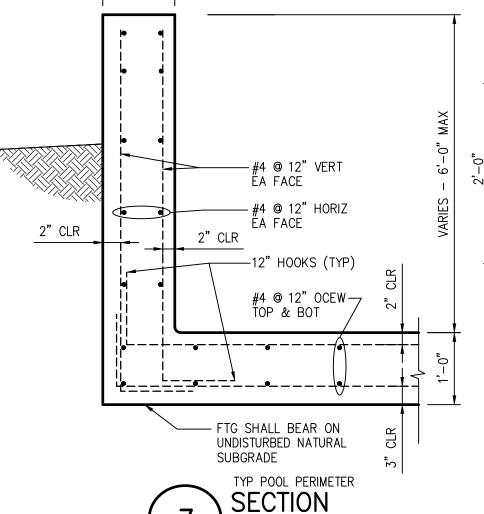


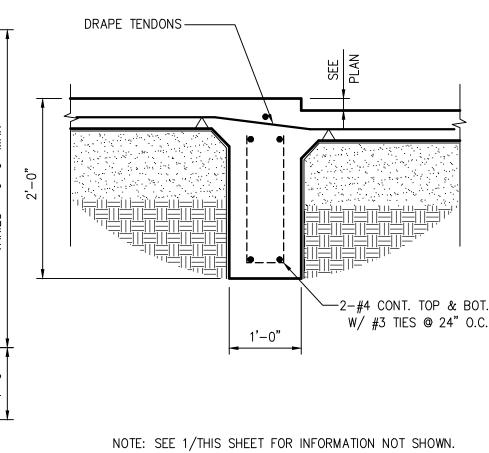
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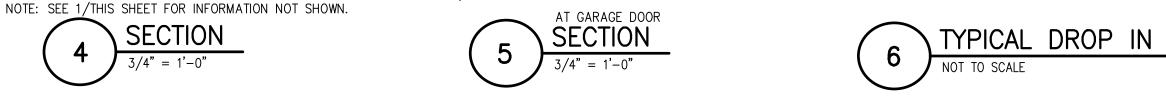






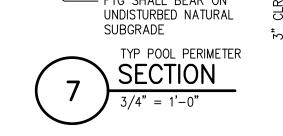


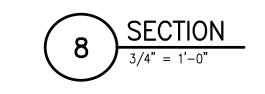


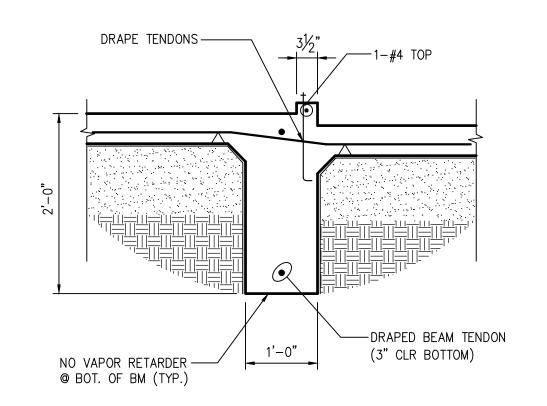


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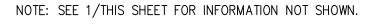


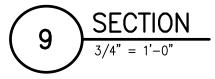


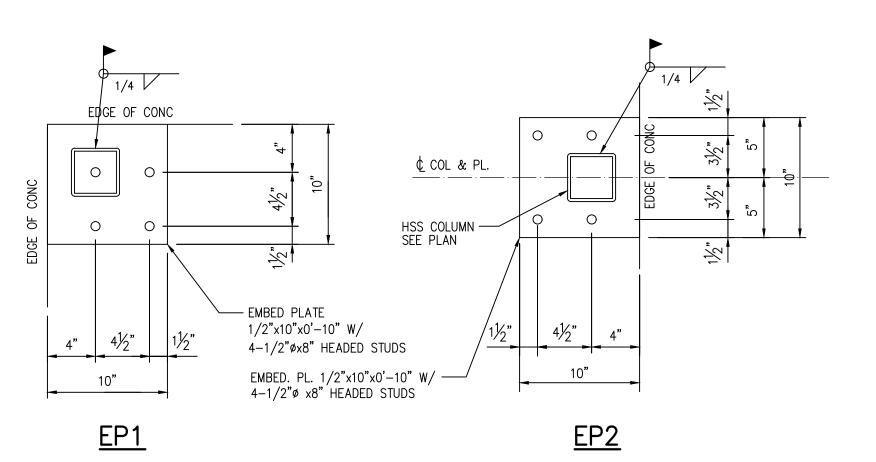
— BEAM REINFORCE

1'-0"

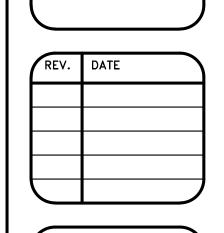
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GREENEARTH ENGINEERING
STRUCTURAL CONSULTING ENGI
2500 WEST WILLAM CANNON DR, #201 AUSTIN,
PHONE (512) 289-8086 FAX (512)

CHK. BY: TZ DRWN. BY: BB DATE: 10/29/2021

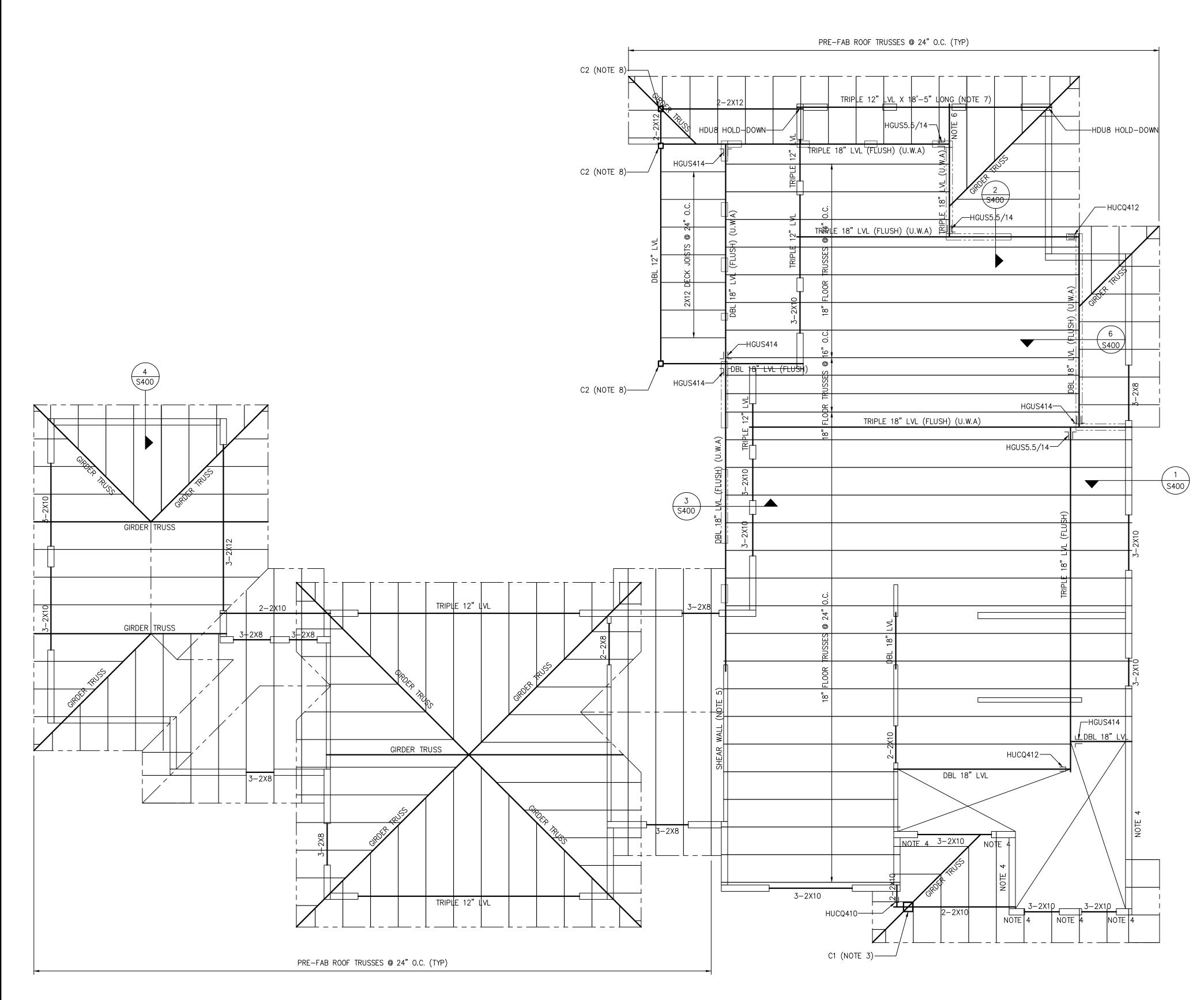
SHEET NO.

S200

GREENEARTH ENGINEERING, INC.
STRUCTURAL CONSULTING ENGINEERS
2500 WEST WILLIAM CANNON DR, #201 AUSTIN, TX 78745
PHONE (512) 289-8086 FAX (512) 383-8339
GE JOB NO.: 21457

CHK. BY: TZ DRWN. BY: BB DATE: 10/21/2021

SHEET NO.



BUILT-UP COLUMN SCHEDULE				
	BUILT-UP STUD COLUMN			
2- 1 3/4"x14" LVL & UP	4- STUD COLUMN			
2- 1 3/4"x12" LVL	3- STUD COLUMN			
3- 2 x	3- STUD COLUMN			
2- 2 x 12	3- STUD COLUMN			
2- 2 x 10 OR SMALLER	2- STUD COLUMN			

2ND FLOOR/LOWER ROOF FRAMING PLAN 1/4" = 1'-0" IN 22X34 1/8" = 1'-0" IN 11X17

PLAN NOTES:

- 1. ALL EXT. WALLS ARE 2X6 @ 16" LOAD BEARING WALLS UNLESS NOTED OTHERWISE IN THIS PLAN.
- ALL INTERIOR WALLS ARE 2X4 @ 16" LOAD BEARING WALLS UNLESS NOTED OTHERWISE IN THIS PLAN.
- SEE ARCH'L FOR 2x6 INTERIOR PLUMBING WALLS.
- 2. BEAMS AND HEADERS SHALL BE SUPPORTED BY BUILT-UP COLUMNS & BLOCK SOLID TO FOUNDATION. SEE SCHEDULE IN THIS SHEET FOR BUILT-UP COLUMN SIZES.
- 3. C1 = 8X8 WOOD COLUMN.
- PROVIDE SIMPSON LTSA12 TIE BETWEEN BEAM AND COLUMN.
- 4. STUDS AT STAIR OPENING NEED TO BE 2X6 AND CONTINUOUS FROM FOUNDATION TO ROOF.
- 5. NOTED INTERIOR SHEAR WALL SHALL BE SHEATHED & HAVE 1/2" POXY BOLTS AT 48" O.C. (EMBED = 4").
- 6. LVL TO BE NOTCHED WHERE IT MEETS ROOF. HOWEVER, MINIMUM DEPTH SHALL BE 6".
- BUILDER TO MAKE SURE TRUSS DEPTH AT WALL IS NO LESS THAN 6".
- 7. SEE 10/S400 FOR LVL MOMENT FRAME CONNECTION DETAIL.
- 8. C2 = HSS4X4X1/4 STEEL COLUMN. SEE 8/S400 FOR WOOD BEAM FACE CONNECT TO STEEL COLUMN DETAIL.



CHK. BY: TZ DRWN. BY: BB DATE: 10/21/2021

SHEET NO.

S301

C1 (NOTE 3)— GIRDER TRUSS GIRDER TRUSS GIRDER TRUSS GIRDER TRUSS

GIRDER TRUSS

ROOF FRAMING PLAN

1/4" = 1'-0" IN 22X34 1/8" = 1'-0" IN 11X17

PLAN NOTES:

C1 (NOTE 3)

5 S400

1. ALL EXT. WALLS ARE 2x6 @ 16" LOAD BEARING WALLS UNLESS NOTED OTHERWISE IN THIS PLAN.

ALL INTERIOR WALLS ARE 2x4 @ 16" LOAD BEARING WALLS UNLESS NOTED OTHERWISE IN THIS PLAN.

SEE ARCH'L FOR 2x6 INTERIOR PLUMBING WALLS.

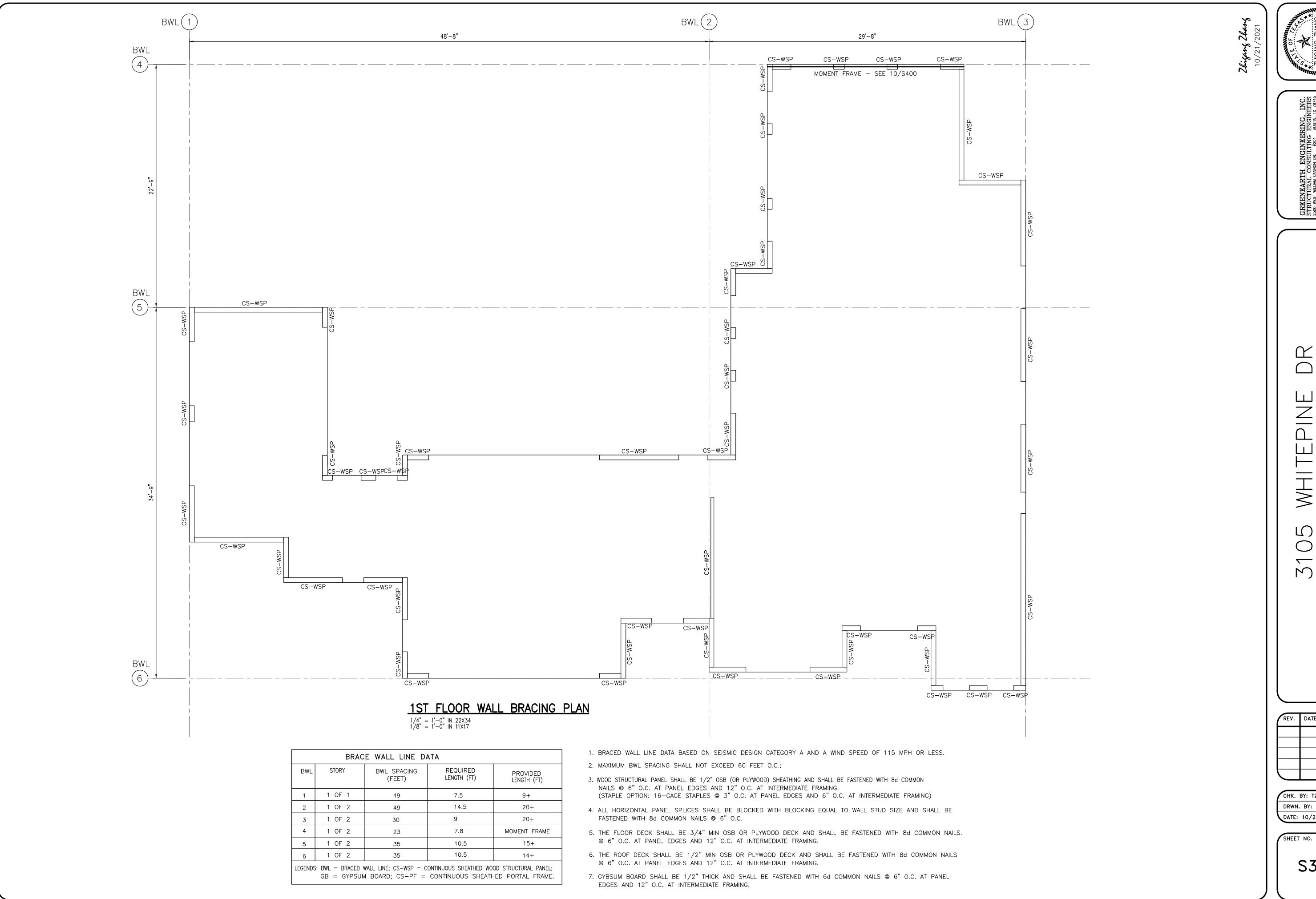
2. BEAMS AND HEADERS SHALL BE SUPPORTED BY BUILT-UP COLUMNS & BLOCK SOLID TO FOUNDATION. SEE SCHEDULE IN THIS SHEET FOR BUILT-UP COLUMN SIZES.

3. C1 = HSS4X4X1/4 STEEL COLUMN. SEE 9/S400 FOR WOOD BEAM OVER STEEL COLUMN CONNECTION DETAIL.

BUILT-UP COLUMN SCHEDULE				
	BUILT-UP STUD COLUMN			
2- 1 3/4"x14" LVL & UP	4- STUD COLUMN			
2- 1 3/4"x12" LVL	3- STUD COLUMN			
3- 2 x	3- STUD COLUMN			

3- STUD COLUMN $2-2 \times 12$ 2- STUD COLUMN

2- 2 x 10 OR SMALLER



CHK. BY: TZ DRWN. BY: BB

DATE: 10/21/2021

S302



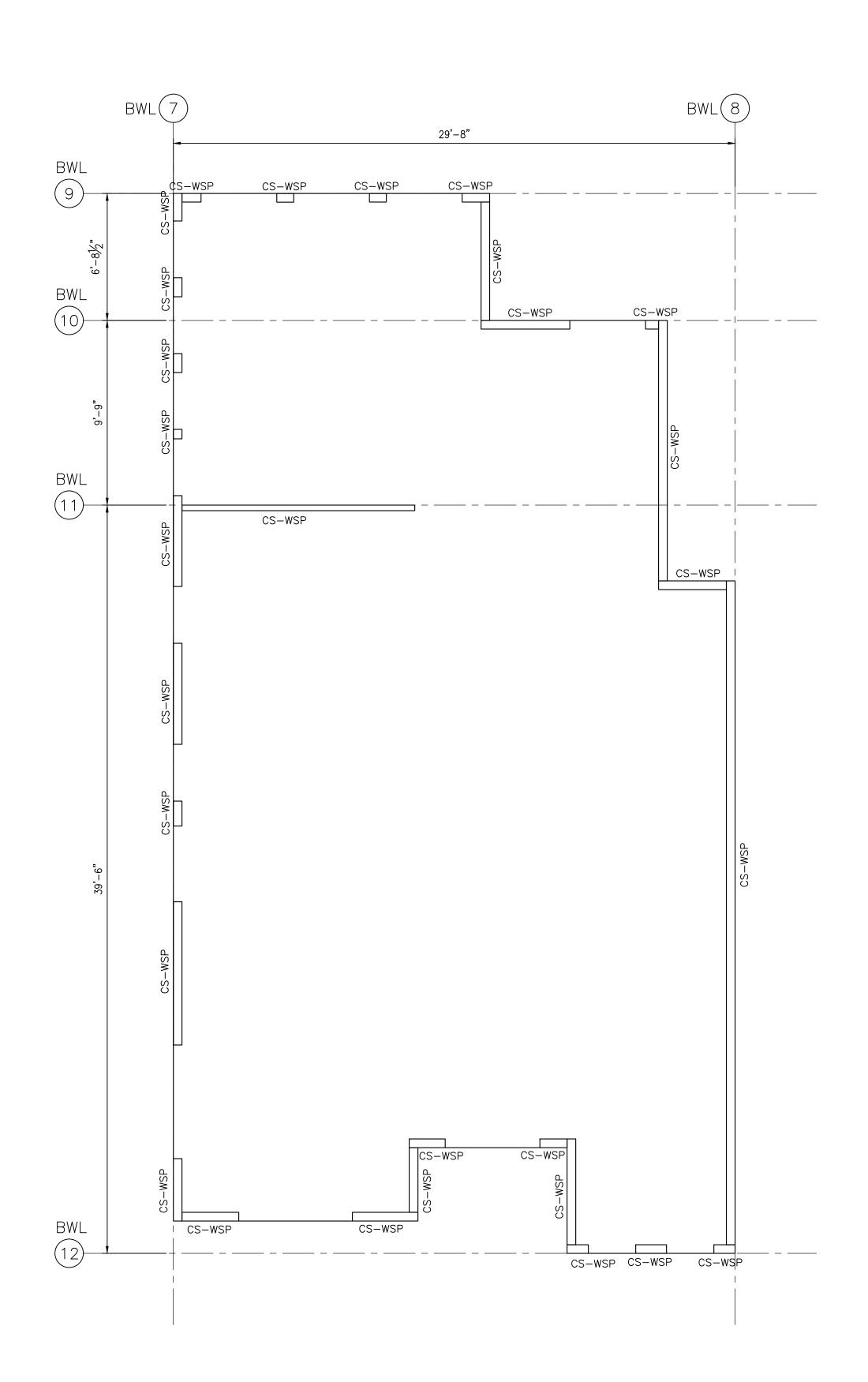


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DATE: 10/21/2021

SHEET NO.

S303



BRACE WALL LINE DATA					
BWL	STORY	BWL SPACING (FEET)	REQUIRED LENGTH (FT)	PROVIDED LENGTH (FT)	
7	2 OF 2	30	5	20+	
8	2 OF 2	30	5	20+	
9	2 OF 2	7	2	3+	
10	2 OF 2	10	2	5+	
11	2 OF 2	40	6	10+	
12	2 OF 2	40	6	10+	
LEGENDS: BWL = BRACED WALL LINE; CS-WSP = CONTINUOUS SHEATHED WOOD STRUCTURAL PANEL; GB = GYPSUM BOARD; CS-PF = CONTINUOUS SHEATHED PORTAL FRAME.					

2ND FLOOR WALL BRACING PLAN 1/4" = 1'-0" | N 22X34 1/8" = 1'-0" | N 11X17

- 1. BRACED WALL LINE DATA BASED ON SEISMIC DESIGN CATEGORY A AND A WIND SPEED OF 115 MPH OR LESS.
- 2. MAXIMUM BWL SPACING SHALL NOT EXCEED 60 FEET O.C.;
- 3. WOOD STRUCTURAL PANEL SHALL BE 1/2" OSB (OR PLYWOOD) SHEATHING AND SHALL BE FASTENED WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE FRAMING. (STAPLE OPTION: 16-GAGE STAPLES @ 3" O.C. AT PANEL EDGES AND 6" O.C. AT INTERMEDIATE FRAMING)
- 4. ALL HORIZONTAL PANEL SPLICES SHALL BE BLOCKED WITH BLOCKING EQUAL TO WALL STUD SIZE AND SHALL BE FASTENED WITH 8d COMMON NAILS @ 6" O.C.
- 5. THE FLOOR DECK SHALL BE 3/4" MIN OSB OR PLYWOOD DECK AND SHALL BE FASTENED WITH 8d COMMON NAILS. @ 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE FRAMING.
- 6. THE ROOF DECK SHALL BE 1/2" MIN OSB OR PLYWOOD DECK AND SHALL BE FASTENED WITH 8d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE FRAMING.
- 7. GYBSUM BOARD SHALL BE 1/2" THICK AND SHALL BE FASTENED WITH 6d COMMON NAILS @ 6" O.C. AT PANEL EDGES AND 12" O.C. AT INTERMEDIATE FRAMING.

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SHEET NO.

S400

STRUCTURAL GENERAL NOTES

- 1. Building Code: 2021 International Residential Code.
- 2. Wood Framing: National Design Specifications For Wood Construction with Supplement, National Forest and Paper
- 3. Structural Plywood: Plywood Design Specification, American Plywood Association, Latest Edition.
- 4. Prefabricated Metal Plate Connected Wood Trusses: Design Standard for Metal Plate Connected Wood Truss Construction, ANSI/TPI Latest Edition.

DESIGN LOADS

- 1. Live Loads
- Roof b. Floor 40 psf c. Stairs

TIMBER FRAMING

- 1. Unless otherwise noted, all structural framing lumber shall be clearly marked no. 2 southern yellow pine or douglas fir, except that non-loadbearing interior walls may be stud grade southern yellow pine, douglas fir, or spruce-pine-fir.
- 2. All wood headers, beams, and top plates shall be no. 2 Southern Yellow Pine or Douglas Fir.
- 3. All load bearing walls shall have solid 2x blocking at 4'-0" o.c. maximum vertically. End nail with 2-16d nails or
- 4. Provide double studs at all wall corners and on each side of all openings, unless noted or detailed otherwise.
- 5. The entire exterior wall framing shall be braced by a 4'-0" wide x 1/2" panel of APA rated sheathing with an exposure 1 rating extending from the top plate to the sill plate. Where wall is taller than 8'-0", provide multiple panels as required to extend from sill plate to top plate. Provide 2x blocking as required to support all panel edges. Nail with 8d common nails at 6" on center at supported edges and 12" on center at intermediate
- 6. Solid 2x blocking or bandboard shall be provided at supports and cantilever ends of all wood joists, and between supports in rows not exceeding 8'-0" apart.
- 7. All framing members framing into the side of a header, hip, valley, ridge, truss or any other beams shall be attached using metal joist hangers manufactured by the Simpson Company or equal. The hanger shall be sized and installed in accordance with the manufacturers recommendations for the size of joist supported.

8. Nailing and attachment of all framing members and sheathing shall be as specified in the International Residential

bolted to the foundation with 1/2" anchor bolts with a minimum embedment of 8" spaced at 4'-0" on center.

Provide a minimum of two bolts per plate segment. Sill plates in contact with concrete or masonry shall be

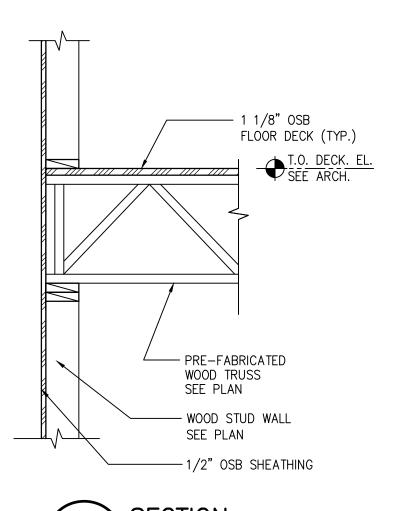
- Code Nailing Schedule (Table R602.3) unless noted otherwise in the drawings. Common wire nails or spikes, or galvanized box nails shall be used for all framing unless noted otherwise. 9. Place a single plate at the bottom and a double plate at the top of all stud walls. Exterior sill plates shall be
- pressure treated with a preservative. 10. Provide double joists under all interior partition walls oriented parallel to the joists.
- 11. Provide triple studs (or cripples) at each end of any header, beam, ridge, valley, or hip spanning over 10'-0" unless noted otherwise. Provide double studs (or cripples) at each end of any header, beam, ridge, valley, or hip spanning 5'-0" to 10'-0" unless noted otherwise.
- 12. The new generation of pressure treated lumber products are highly corrosive to metal connectors and fasteners. All fasteners and metal connectors used in conjunction with the new generation of pressure treated lumber shall be hot-dip galvanized or stainless steel. These locations include, but are not limited to the following:
 - Anchor bolts at sole plate to foundation.
 - Nails from sole plate to wall studs. Nails at exterior plywood sheathing to sole plate.
- Bolts at ledger to concrete. Joist to treated ledger connection
- All hangers on treated joists.
- Wood posts to concrete. Deck board to treated joists.

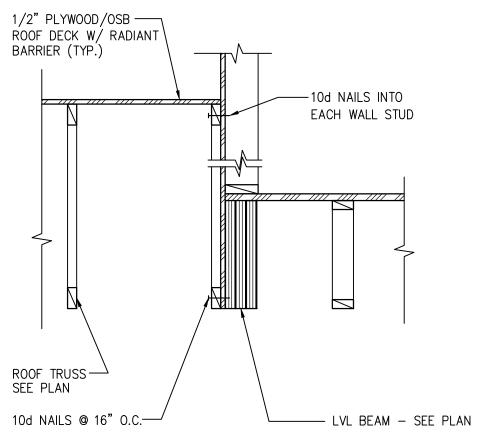
PREFABRICATED METAL PLATE CONNECTED WOOD TRUSSES

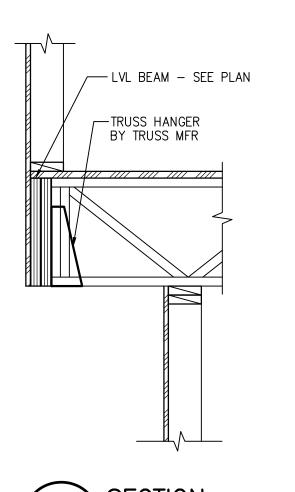
- 1. Trusses shall be designed by the Contractor in accordance with the Truss Plate Institute "Design Standard for Metal Plate Connected Wood Truss Construction" (ANSI/TPI 1-95).
- 2. Truss members shall be clamped in a mechanical or hydraulic jig with sufficient pressure to bring members into reasonable contact at all joints during application of connector plates.
- 3. Provide adequate erection bracing in accordance with Truss Plate Institute publication HIB-91.
- 4. Truss Manufacturer shall provide permanent bracing as required by the design of the trusses. Erection bracing may remain in place as permanent bracing where it does not interfere with the architectural finishes.
- 5. All timber truss members shall be Southern Yellow Pine with a maximum moisture content of 19%. Chord members shall be no. 2 or better and web members shall be no. 3 or better.
- 6. Connection plates shall be manufactured by a WTCA member plate manufacturer. Plates shall be 20 gauge minimum, ASTM A446 grade A steel, with a G60 galvanized coating.
- 7. Trusses shall be designed in accordance with the following requirements:
- a. Top chords shall be designed to resist the local bending induced by the floor or roof uniform load on the
- b. Limit live load deflection of floor trusses to L/480. Total load deflections shall be limited to L/360.

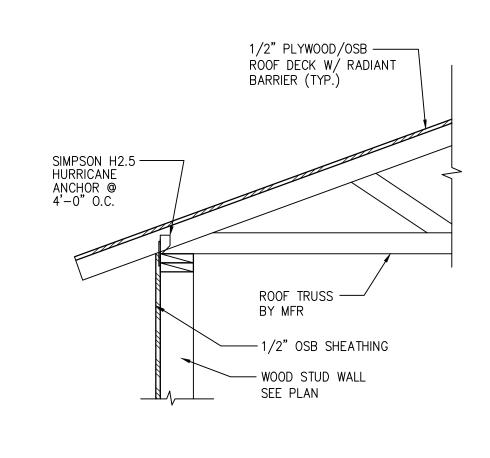
COMPOSITE WOOD MEMBERS

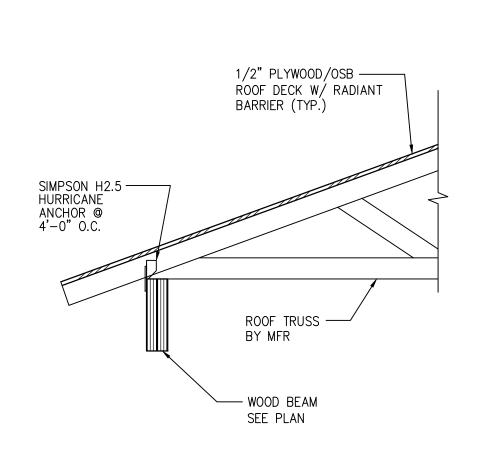
- 1. Where noted on the drawings, joists shall be TJI "SP" series engineered wood joists, and beams shall be "Micro—Lam" or "Parallam" beams as manufactured by the Trus Joist Macmillan Corporation.
- 2. Do not notch joists or beams. Drill holes through webs of engineered wood members for mechanical, electrical or plumbing services in accordance with the recommendations of the engineered wood product manufacturer.
- 3. Multiple wood beams up to three members thick shall be nailed together with three rows of 16d nails at 12" on center. Four or more multiple wood beams and any multiple wood beams utilizing beams thicker than 1 3/4" shall be bolted together with 1/2" diameter bolts top and bottom at supports and ends of the beam, then at 24" on center, staggered top and bottom for the full length of the beam.
- 4. Where multiples of two 1 3/4" Micro-Lam beams are noted on the drawings, contractor may provide single 3 1/2" beams in lieu of double 1 3/4" beams.
- 5. Provide web stiffeners where required by the manufacturer for the specified support condition.

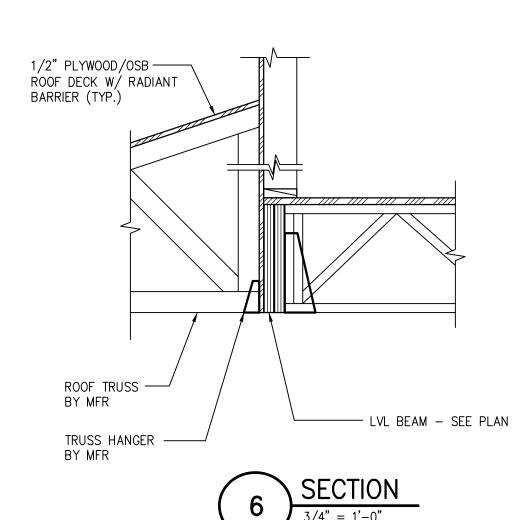






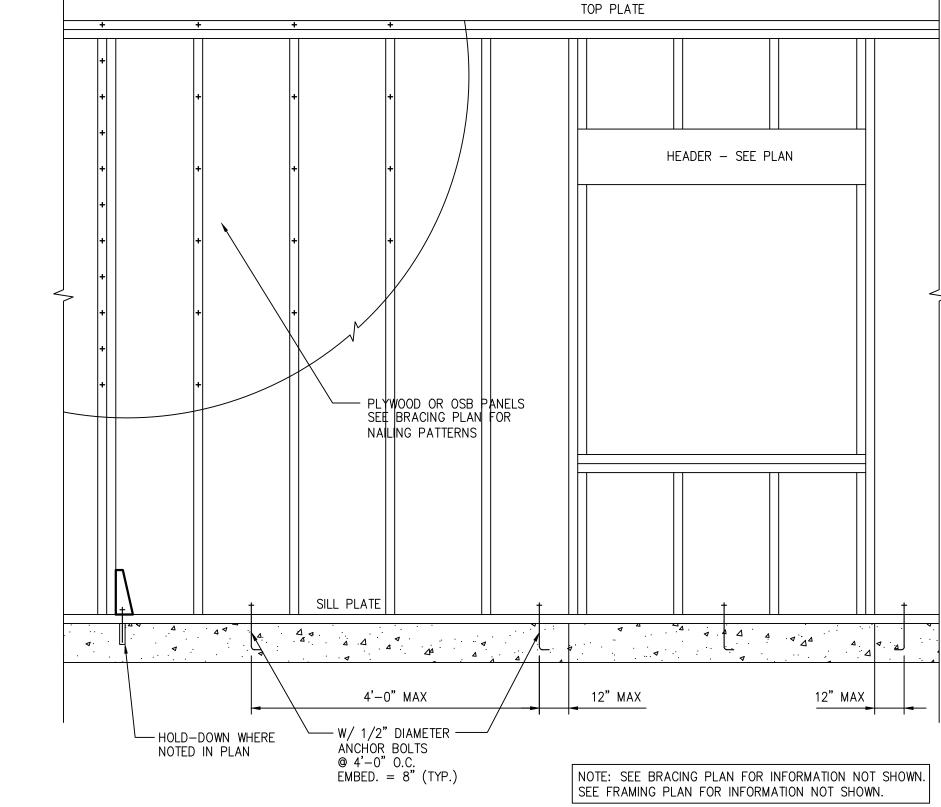




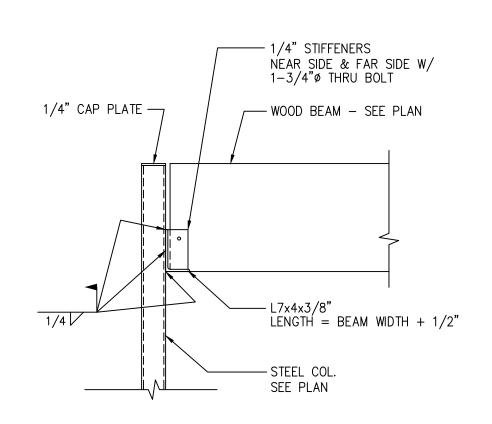


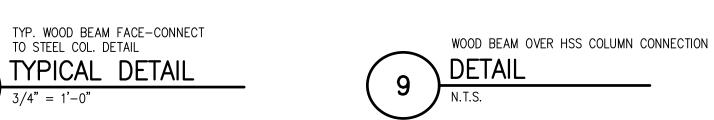
— STEEL PLATE 1/4"x4"x10" W/

 $2^{\prime}-1/2^{\prime\prime}$ ø x 4 $^{\prime\prime}$ LAG SCREWS



ELEVATION VIEW





WOOD BEAM-SEE PLAN

CAP PLATE-1/4"x4"x4"

STL. COL.— SEE PLAN

